

## Article

# Investigation of Dynamic Behavior of Ultra-Large Cold-Water Pipes for Ocean Thermal Energy Conversion

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**Abstract:** Ocean Thermal Energy Conversion (OTEC) is a process that can produce electricity by utilizing the temperature difference between deep cold water and surface warm water. The cold-water pipe (CWP) is a key component of OTEC systems, which transports deep cold water to the floating platform. The CWP is subjected to various environmental and operational loads, such as waves, currents, internal flow, and platform motion, which can affect its dynamic response and stability. In this paper, we establish a computational model of the mechanical performance of the CWP based on the Euler–Bernoulli beam theory and the Morrison equation, considering the effects of internal flow, sea current, and wave excitation. We use the differential quadrature method (DQM) to obtain a semi-analytical solution of the lateral displacement and bending moment of the CWP. We verify the correctness and validity of our model by comparing it with the finite element simulation results using OrcaFlex software. We also analyze the effects of operating conditions—such as wave intensity, clump weight at the bottom, and internal flow velocity—on the dynamic response of the CWP using numerical simulation and the orthogonal experimental method. The results show that changing the wave strength and internal flow velocity has little effect on the lateral displacement of the CWP but increasing the current velocity can significantly increase the lateral displacement of the CWP, which can lead to instability. The effects of waves, clump weight, internal flow, and sea current on the maximum bending moment of the CWP are similar; all of them increase sharply at first and then decrease gradually until they level off. The differences in the effects are mainly reflected in the different locations of the pipe sections. This paper suggests some design guidance for CWP in terms of dynamic responses depending on the operating conditions. This paper contributes to the journal’s scope by providing a novel and efficient method for analyzing the mechanical performance of CWP for OTEC systems, which is an important ocean energy resource.

**Keywords:** ocean thermal energy conversion; ultra-large cold-water pipe; mechanical properties; numerical simulation; sensitivity analysis; orthogonal experimental method



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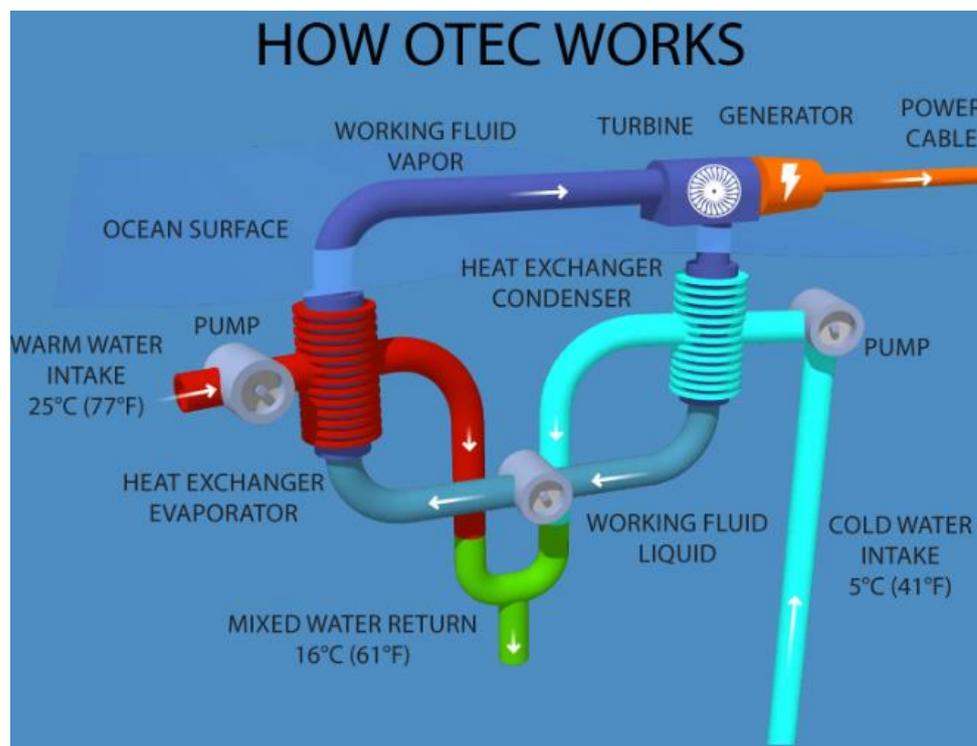


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## 1. Introduction

With petroleum resource production peaking and the demand increasing, humanity will face a steadily diminishing supply of fossil fuels. It seems sensible to consider ocean energy resources as some of the replacement “fuels” for future energy technologies. Ocean thermal resources, as one of the three major ocean energies, is always available, while all other ocean energy resources are intermittent. Ocean Thermal Energy Conversion (OTEC) is a process that can produce electricity by utilizing the temperature difference between deep cold water and surface warm water, as shown in Figure 1 [1]. The cold-water pipe (CWP) is a novel and most challenging component of floating structures for OTEC, which

is used to transport deep cold water to floating platforms [2]. For commercial-scale use, the CWP is as large as 12 m in diameter and up to 1000 m in length [3]. Ultra-large cold-water pipes will be subjected to wind, waves, currents, collisions, and platform movements in the complex marine environment. These internal and external factors can directly affect the dynamic response of CWP and the safety of in situ operations. To ensure the safe and smooth operation of CWP, it is necessary to analyze the dynamic response of the pipe in the marine environment in order to obtain reliable prediction results.



**Figure 1.** Ocean Thermal Energy Conversion operating principle.

Since the concept of a chilled water pipe was first proposed to be applied in OTEC systems, the study of the dynamic performance of chilled water pipes has been a key research object in OTEC installations. As a typical marine riser, the design of cold-water pipes can be divided into three main issues [4]: strength analysis, which includes limit analysis and crush analysis; cold-water pipe-platform coupling analysis; and cold-water pipe vibration analysis, which includes internal flow excitation and vortex excitation vibration. When Kuiper et al. [5] investigated the stability of suspended flow transfer pipes, they found errors between theoretical predictions and experiments, which were caused by the incorrect description of the negative pressure of the pipe. Halkyard et al. [6] summarized the research work on cold-water pipes in the past three decades (2014) and found that the research work mainly focused on the analysis of the dynamic performance of cold-water pipes under the action of waves, currents, and platform motion, and little consideration was given to the effect of instream flow. Adiputra [2] focused on the effects of pipe material, top connection, and bottom support system on the dynamic performance of cold-water pipes. Additionally, the self-induced vibration of the cold-water pipe was analyzed and solved numerically. The results showed that the critical flow velocity predicted in the time domain was on average 20% higher compared to the frequency domain, and the clump weight at the bottom under the lighter material had a more significant effect on the critical flow velocity compared to the relatively high-density material [3]. Shan [7] established an OTEC hull cold-water pipe coupling analysis and benchmarked the cold-water pipe through a coupling analysis software program. Halkyard and their associates [8] determined the dynamic strain imparted on the cold-water pipe due to waves and currents

through simulations using the offshore dynamic analysis program HARP; this was verified by Makai Offshore using OrcaFlex software, which showed that the maximum pipe strain was within 25% of the HARP prediction. Xiang [9] based a study on the cold-water pipe vibration problem. Thirugnana et al. [10] used the steepest descent method to optimize the design parameters of the OTEC system, such as the evaporation and condensation temperatures, the seawater velocities, and the heat transfer area. The article also discusses the effects of different inlet temperatures and cold seawater pipe lengths on the net power output and the objective function. The dynamic response of the cold-water pipe to extreme conditions was investigated, and the vortex-induced vibration response and other design issues of the cold-water pipe were discussed.

As a typical conveying fluid riser, much research work has been conducted by domestic and foreign scholars on the vibration problem of the pipe. Aitken [11] first experimentally illustrated the equilibrium between tension and centrifugal forces induced by motion in 1876, which was the earliest work that studied the dynamics of pipes conveying fluids. In 1939, Bourrieres [12] studied the oscillatory instability of cantilevered pipes conveying fluids theoretically and experimentally, deriving the equations of motion of the pipes. Housner [13] found that the internal flow velocity significantly affects the pipe's buckling. Gregory and Paidoussis [14,15] demonstrated theoretically and experimentally that cantilevered pipes undergo oscillatory instability (chattering) at sufficiently high flow velocities. Benjamin [16] found that vertical cantilevered systems subjected to gravity are susceptible to buckling instability. Wu et al. [17] used the micro-element method to establish the equations of motion of a suction pipe in still water. Based on the finite unit method, Guo [18] and Li [19] studied the static and dynamic characteristics of a riser containing internal flow. Based on the above-related studies, it can be seen that a large number of scholars have done a lot of research work on the dynamic pipeline response problem, while the current research on the dynamic response of the pipe has not considered the problem of the fluid inside the pipe. Meanwhile, the external flow field (sea current and waves) has not been explored in depth during the study of the fluid transfer pipe. Therefore, studying the dynamic response under the collective excitation of internal flow, sea current, and waves must be studied in depth.

Using analytical methods to predict the dynamic response behavior of cold-water pipes can provide a clearer insight into the nature of the problem. At the same time, the analytical method can accurately and quickly extract the influence of critical parameters on the structural dynamic behavior of chilled water pipes efficiently and rapidly, thus guiding the design of cold-water pipes. When using the analytical method to study the dynamic response of the pipe, it is usually desirable to consider the mechanical properties of the pipe comprehensively in the calculation. However, the difficulty of the calculation is always proportional to the comprehensiveness of the mechanical properties of the pipe. Therefore, in most cases, the analysis is focused on a single mechanical property of the pipe or a single boundary condition. The commonly used analytical methods in dealing with the relevant dynamic pipe response problems are the Galerkin method [20–23], Finite Element Method (FEM) [24,25], Finite Difference Method (FDM) [26–29], and the Transfer (Matrix Method) [30,31]. The above methods have the disadvantages of computational limitations and complexity.

This paper uses a semi-analytical method that can accurately and quickly solve high-order nonlinear partial differential equations. Liang [32] et al. studied the effect of pipe end boundary conditions and geometric parameters on pipe displacements using the differential product (DQM) method. The results showed that the displacement increases with the increase in pipe degrees of freedom, increases with the increase in length/diameter ratio, and decreases with the increase in thickness/diameter ratio. An and Su [33,34] used the generalized integral transformation (GITT) method to derive a semi-analytical solution for the vibration of a flow transport pipe due to two-phase flow under solid-supported boundary conditions and analyzed the effect of gas volume fraction and volume flow rate in a two-phase flow on the dynamic behavior of the pipe. Li [35] et al. used the GITT

method to analyze the effect of different internal and external flow velocities and different soil stiffness on the dynamic response of a free-hanging span pipe. Li [36] applied the GITT method to study the effect of damping ratio on the dynamic behavior of a pipe under simply supported boundary conditions and verified it by comparing it with existing experimental results to obtain dimensionless structural parameters that can control the standardized stability envelope of the pipe. Qiao et al. [37] studied the bifurcation and chaotic motion of fluid transport bends under nonlinear constraints based on the DQM method. The discrete form of the system's equations of motion was established and solved numerically. Huang [38] et al. used the differential quadrature method (DQM) to investigate the effects of various design parameters, such as beam shape, electrode position, and tip thickness, on the dynamic response and settle time of the microswitch. Liang [39] et al. employed the Laplace transform, Fourier transform, differential quadrature method, and state space method to derive the solutions for the displacement, stress, electric potential, and dielectric displacement of the laminates. The article also considers the fluid–structure interaction and different boundary conditions for the laminates.

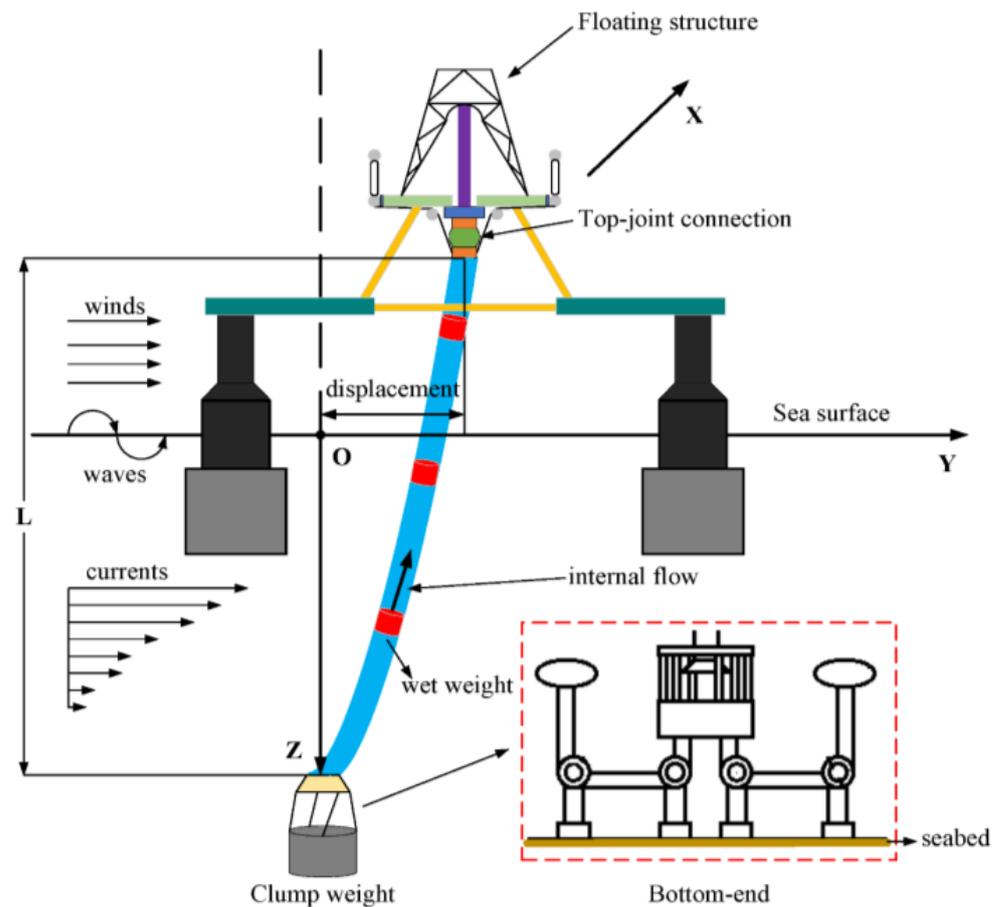
In this paper, based on the Euler–Bernoulli beam theory and Morrison equation theory, we establish the lateral motion control equation of large-diameter cold-water pipes and consider the effect of internal flow and current and wave excitation, comprehensively, and we use the DQM method to obtain its semi-analytical solution. In comparison with the finite element simulation results of Orcaflex, the correctness of the semi-analytical solution is verified, and the effects of the wave, current velocity, internal flow velocity, and the clump weight on the dynamic response characteristics of the large-diameter cold-water pipes are analyzed by combining them with the orthogonal experimental method. The main parameter variables in the paper are shown in Table 1.

**Table 1.** The main parameters of the CWP.

Nomenclature	Description
$EI$	Bending stiffness (N/m <sup>2</sup> )
$L$	Pipe length (m)
$M_p$	Mass of the internal flow per unit length (kg/m)
$M_f$	Mass of the pipe per unit length (kg/m)
$T$	Axial equivalent tension (N)
$U$	Velocity of the internal flow (m/s)
$\phi(x, t)$	Transverse displacement of the pipe (m)
$A_f$	Internal cross-sectional areas (m <sup>2</sup> )
$A_0$	External cross-sectional areas (m <sup>2</sup> )
$\rho_w$	Density of the seawater (kg/m <sup>3</sup> )
$C_a$	Added mass coefficient
$P_i$	Internal pressure of the pipe
$P_0$	External pressure of the pipe
$C_d$	Adapted drag coefficient

## 2. Analytical Simulation

The analytical model of the cold-water pipe shown in Figure 2 is established and simplified as a Euler–Bernoulli beam. The cold-water pipe is a vertical cantilevered pipe, which is restrained at the upper end by articulation and at the lower end by a counterweight block, and it carries deep cold water upward from a water depth of 1200 m. The top of the cold-water pipe is subjected to the top tension applied by lifting and sinking compensator–tensioner. Before analyzing the cold-water pipe, reasonable assumptions are required [37].



**Figure 2.** Schematic diagram of load of CWP system.

- (1) Assume that the cold-water pipe is composed of homogeneous, isotropic, and viscoelastic material, ignoring the effect of the connecting joint between the pipes.
- (2) Meet the Euler–Bernoulli theory model, ignoring the effect of axial shear.
- (3) The incompressible fluid inside and outside the pipe.

The calculation of hydrodynamic loads is generally performed using the Morrison equation, the rounding theory, and the Froude–Krylov assumption. Since the structures in this paper are slender marine structures, the Morrison equation is used for the calculation. The basic idea of the Morrison equation will divide the forces of waves and currents on the pipe into drag and inertia forces [40]. The use of the Morrison equation for hydrodynamic load calculations requires reasonable assumptions.

- (4) The ratio of the diameter of the member to the wavelength is not greater than 0.2, i.e.,  $D/L \leq 0.2$ .

- (5) The member is rigid, has a smooth surface, and is perpendicular to the seafloor.

According to the analysis of the engineering condition of the temperature difference for the energy cold-water pipe, the load acting on the pipe system includes two categories of environmental and operational load. Environmental loads include waves, currents, and sea wind loads; operational loads include the gravity and buoyancy of the pipe itself, hydrodynamic loads, the gravity of counterweight blocks, internal fluid loads, etc.

Figure 2 shows the length of the cold-water pipe for  $L$  uniform pipe, the internal perimeter  $S$ , the internal cross-sectional area of the pipe  $A_f$ , the external cross-sectional area of the pipe  $A_o$ , the mass of the fluid inhaled per unit length  $M_f$ , the mass of the pipe per unit length  $M_p$ , the bending stiffness of the pipe  $EI$ , the intra-pipe flow rate of the fluid in the pipe  $U$ , and the uniform cross-section of the pipe. The pipe is in an equilibrium position along the  $x$ -axis when no vibration occurs, and the effect of gravity is not negligible. The flow velocity may be affected by minor external disturbances, so

generally  $dU/dt \neq 0$ . Considering that the pipe is a slender structure, its lateral motion is small, and its wavelength is long, relative to its diameter; therefore, the curvilinear coordinates  $s$  and  $x$  along the centerline of the pipe can be used interchangeably according to the previous discussion. Consider both the fluid and the pipe microelements, as shown in Figures 3 and 4.

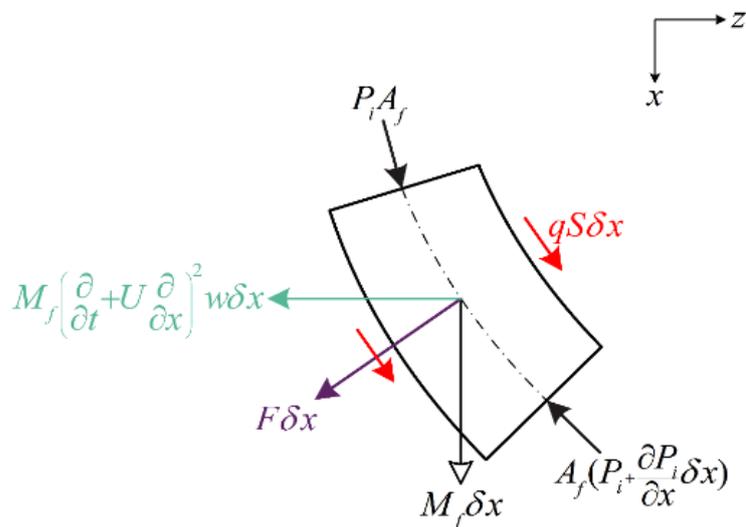


Figure 3. Force analysis diagram of fluid elements.

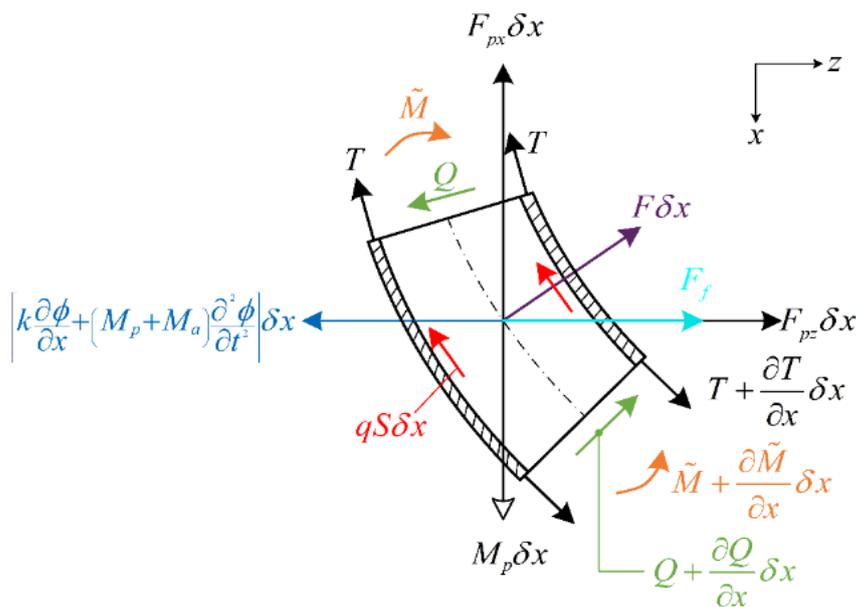


Figure 4. Pipe element force analysis diagram.

As shown in Figure 3, the fluid micro-element is subjected to axial pressure ( $P = P(x, t)$ ), the internal hydrostatic pressure ( $P_i$ ), and the external hydrostatic pressure ( $P_o$ ) on the cold-water pipe, respectively. Figure 3 also shows the fluid force tangential to the fluid micro-element ( $qS\delta x$ ); the average fluid force perpendicular to the fluid element ( $F\delta x$ ) (the reaction force of the pipe on the fluid); the axial, lateral, centrifugal, and kurtosis inertia forces of the fluid itself; and gravity ( $M_f\delta x$ ). According to Newton’s second law of motion and the Euler–Bernoulli beam theory, the pipe has a slight deformation, neglecting the second-order term of acceleration of the pipe in the  $x$ -direction, as well as transverse shear deformation and rotational inertia, and the fluid unit Axial direction force equilibrium equation is as follows.

$$-A_f \frac{\partial P_i}{\partial x} + qS + M_f g + F \frac{\partial \phi}{\partial x} + M_f \frac{\partial U}{\partial t} = 0 \tag{1}$$

The force balance equation in the z-axis direction of the fluid unit is as follows [14].

$$F + M_f \left( \frac{\partial}{\partial t} + U \frac{\partial}{\partial x} \right)^2 \phi + A_f \frac{\partial}{\partial x} \left( P_i \frac{\partial y}{\partial x} \right) - qS \frac{\partial \phi}{\partial x} - M_f \frac{\partial U}{\partial t} \frac{\partial \phi}{\partial x} = 0 \tag{2}$$

The pipe micro-element is subjected to axial tension perpendicular to the pipe cross-section ( $T$ ), transverse shear force in the pipe ( $Q$ ), bending moment ( $\tilde{M}$ ), normal ( $F\delta x$ ) and tangential ( $qS\delta x$ ) forces of the fluid on the pipe, gravity ( $M_p\delta x$ ), hydrostatic pressure of the fluid around the pipe in any direction, external fluid forces on the pipe ( $F_f$ ), forces generated by the additional mass of the pipe and surrounding fluid ( $(M_p + M_a) \frac{\partial^2 \phi}{\partial x^2} \delta x$ ), and forces generated by friction-related damping ( $k \frac{\partial \phi}{\partial x} \delta x$ ). According to Newton’s second law and the Euler–Bernoulli beam theory, the pipe has a slight deformation, ignoring the second-order acceleration term in the direction of the pipe, as well as transverse shear deformation and rotational inertia, for the pipe unit in the  $x$ -axial and  $z$ -axial directions, respectively. The force balance equations and moment balance equations are shown in Figure 4.

The equation for the balance of forces in the  $x$ -axial direction is as follows.

$$\frac{\partial T}{\partial x} - qS + M_p g - F_{px} - F \frac{\partial \phi}{\partial x} = 0 \tag{3}$$

The equation for the balance of forces in the  $z$ -axial direction is as follows.

$$\frac{\partial Q}{\partial x} + F - (M_f + M_a) \frac{\partial^2 \phi}{\partial t^2} - k \frac{\partial \phi}{\partial t} + \frac{\partial}{\partial x} \left( T \frac{\partial y}{\partial x} \right) - qS \frac{\partial \phi}{\partial x} + F_{pz} + F_f = 0 \tag{4}$$

The moment balance equation for the micro-element column is as follows.

$$Q = -\frac{\partial \tilde{M}}{\partial x} \tag{5}$$

The pipe is a viscoelastic material subject to Kelvin–Voigt [41,42]-type internal dissipation and follows the following stress–strain relationship.

$$\sigma = E\varepsilon + E^* \frac{\partial \varepsilon}{\partial x} = \left( E + aE \frac{\partial}{\partial x} \right) \varepsilon \tag{6}$$

where  $E$  is the modulus of elasticity of the pipe material,  $E^*$  is the dissipation coefficient of the pipe material, and  $a$  is the viscoelastic coefficient of the pipe. From Equation (6), the bending moment  $\tilde{M}$  can be expressed as follows.

$$\tilde{M} = \left( E + E^* \frac{\partial}{\partial x} \right) I \frac{\partial^2 \phi}{\partial x^2} \tag{7}$$

Substituting (7) into (5) yields

$$Q = -\left( E + E^* \frac{\partial}{\partial x} \right) I \frac{\partial^3 \phi}{\partial x^3} \tag{8}$$

Considering the fluid as incompressible, and neglecting the temperature effect of the fluid, there is a simple linear relationship between the pressure in hydrostatic water [43,44].

$$P_o(x) = P_{oL} \frac{x}{L} \tag{9}$$

It can therefore be concluded that

$$F_{px} = 0, F_{pz} = \frac{\partial}{\partial x} \left( P_o A_o \frac{\partial \phi}{\partial x} \right) \tag{10}$$

Taking Equation (9) into (10) gives

$$F_{pz} = \frac{A_o P_{oL}}{L} \frac{\partial \phi}{\partial x} + \frac{A_o P_{oL} x}{L} \frac{\partial^2 \phi}{\partial x^2} \tag{11}$$

By adding up Equations (1) and (3) and substituting into (10), we obtain

$$\frac{\partial}{\partial x} (T - P_i A_f) + (M_f + M_p)g + M_f \frac{\partial U}{\partial t} = 0 \tag{12}$$

The additional axial force generated by the pressure on the fluid at the lowermost end of the pipe is  $P_{0L}(1 - 2\nu\delta)$ . The axial force  $\delta$  at the end of the pipe is equal to 1 when the pipe is restrained and 0 when it is free. Secondly, the axial force at the pipe is generated with the gravity of the counterweight minus its buoyancy, i.e.,

$$\bar{T} = \bar{M} - F_b \tag{13}$$

Integrating from (12) gives

$$(T - P_i A_f)_x = \bar{T} - P_{oL} A_f (1 - 2\nu\delta) + [(M_f + M_p)g + M_f \frac{\partial U}{\partial t}](L - x) \tag{14}$$

Subtracting from Equations (2) and (4) gives

$$-\frac{\partial Q}{\partial x} + M_f \left( \frac{\partial}{\partial x} + U \frac{\partial}{\partial x} \right)^2 w + \frac{\partial}{\partial x} \left( (T - A_f P_i) \frac{\partial \phi}{\partial x} \right) + (M_f + M_a) \frac{\partial^2 \phi}{\partial t^2} + k \frac{\partial \phi}{\partial t} - M_f \frac{\partial U}{\partial t} \frac{\partial \phi}{\partial x} - F_{pz} - F_f = 0 \tag{15}$$

Taking (8), (10), (11), and (14) into the above equation yields

$$\begin{aligned} & \left( E + E^* \frac{\partial}{\partial x} \right) I \frac{\partial^4 \phi}{\partial x^4} + \left\{ M_f U^2 - \bar{T} + P_{oL} A_f (1 - 2\nu\delta) \right. \\ & \left. - \left[ (M_f + M_p)g + M_f \frac{\partial U}{\partial t} \right] (L - x) - \frac{A_o P_{oL} x}{L} \right\} \frac{\partial^2 \phi}{\partial x^2} \\ & + \left[ (M_f + M_p)g - \frac{A_o P_{oL}}{L} \right] \frac{\partial \phi}{\partial x} + 2M_f U \frac{\partial^2 \phi}{\partial t \partial x} \\ & + (M_f + M_p + M_a) \frac{\partial^2 \phi}{\partial t^2} + k \frac{\partial \phi}{\partial t} - F_f = 0 \end{aligned} \tag{16}$$

According to the Morison [40] equation, the total drag force acting on the vertical column structure per unit length can be obtained as

$$F_D = \frac{1}{2} C_D \rho_w D_O V |V| \tag{17}$$

where  $F_D$  is the horizontal drag force and  $\rho_w$  is the drag force coefficient. When the Reynolds number  $Re < 3 \times 10^5$ ,  $C_D = 1.2$ .  $\rho_w$  is the seawater density,  $D_O$  is the riser outer diameter, and  $V$  is the current flow velocity.

The inertial force acting on a vertical cylindrical structure per unit length can be expressed as

$$F_I = \rho_w \frac{\pi D^2}{4} \frac{\partial V}{\partial t} + C_a \rho_w \frac{\pi D^2}{4} \frac{\partial V}{\partial t} = C_m \rho_w \frac{\pi D^2}{4} \frac{\partial V}{\partial t} \tag{18}$$

where  $C_a$  is the additional mass coefficient,  $C_a = 1.2$ ,  $C_m$  is the inertia force coefficient ( $C_m = C_a + 1$ ).

Then, the hydrodynamic load acting on the vertical cylindrical structure per unit length can be obtained as

$$F_f = F_D + F_I = \frac{1}{2} C_D \rho_w D_O V |V| + C_m \rho_w \frac{\pi D^2}{4} \frac{\partial V}{\partial t} \tag{19}$$

In summary, combining Equations (16) and (19) yields the differential equations of motion for the pipeline under internal flow, sea current, and wave excitation as

$$\begin{aligned} & \left( E + E^* \frac{\partial}{\partial x} \right) I \frac{\partial^4 \phi}{\partial x^4} + \left\{ M_f U^2 - \bar{T} + P_{oL} A_f (1 - 2\nu\delta) \right. \\ & \left. - \left[ (M_f + M_p)g + M_f \frac{\partial U}{\partial t} \right] (L - x) - \frac{A_o P_{oL} x}{L} \right\} \frac{\partial^2 \phi}{\partial x^2} \\ & + \left[ (M_f + M_p)g - \frac{A_o P_{oL}}{L} \right] \frac{\partial \phi}{\partial x} + 2M_f U \frac{\partial^2 \phi}{\partial t \partial x} \\ & + (M_f + M_p + M_a) \frac{\partial^2 \phi}{\partial t^2} + k \frac{\partial \phi}{\partial t} - \frac{1}{2} C_D \rho_w D_O V |V| - C_m \rho_w \frac{\pi D^2}{4} \frac{\partial V}{\partial t} = 0 \end{aligned} \tag{20}$$

Assuming that the pipe boundary conditions are simply supported at both ends, the expressions are

$$\begin{aligned} EI\phi'''(0) &= -K_1\phi(0), EI\phi''(0) = K_{T1}\phi'(0) \\ EI\phi'''(L) &= K_2\phi(L), EI\phi''(L) = -K_{T2}\phi'(L) \end{aligned} \tag{21}$$

where  $K_1, K_2$  is the top and bottom linear spring stiffness, respectively, and  $K_{T1}, K_{T2}$  is the top and bottom torsional spring stiffness, respectively. In this paper, the boundary conditions of the water pipe include the following: the top is supported, i.e., the top displacement and bending moment are 0; the bottom is a constraint with a concentrated mass ( $\bar{M}$ ), i.e., the bottom bending moment is equal to 0. A mathematical formula can express the boundary conditions as

$$\begin{aligned} \phi(0) = \phi''(0) = 0, \phi''(L) = 0 \\ EI\phi'''(L) = \bar{M}\omega^2\phi(L) \end{aligned} \tag{22}$$

where  $K_1 \rightarrow \infty, K_{T1} = 0, K_2 = \bar{M}\omega^2, K_{T2} = 0$ , and  $a^4 = \frac{\omega^2\bar{M}}{EI}$ .

The differential equation of motion in the cold-water pipe is simplified by introducing dimensionless coefficients, which are as follows.

$$\begin{aligned} \xi &= \frac{x}{L}, \eta = \frac{\phi}{L}, \tau = \sqrt{\frac{EI}{M}} \frac{t}{L^2}, u = \sqrt{\frac{M_f}{EI}} UL, \beta = \frac{M_f}{M_p}, \\ \gamma &= \frac{(M_p + M_f)L^3}{EI} g, \zeta = \frac{P_o L A_o L^2}{EI}, \Gamma = \frac{(\bar{M} - F_b)L^2}{EI}, \\ \Pi &= \frac{P_o L A_f L^2}{EI}, \alpha = \sqrt{\frac{I}{EM_p}} \frac{E^*}{L^2}, d = \frac{D_o}{L}, \\ v &= \sqrt{\frac{M_p}{EI}} VL, w = \sqrt{\frac{M_p}{EI}} w_s L^2, \rho = \frac{L^2}{M_p} \rho_w, \\ k_1 &= \frac{L^3}{EI} K_1, k_2 = \frac{L^3}{EI} K_2, k_{T1} = \frac{L}{EI} K_{T1}, k_{T2} = \frac{L}{EI} K_{T2} \end{aligned} \tag{23}$$

The primary consideration in this study is that the internal fluid is a constant flow, i.e., for the stability problem of self-excited vibration, so Equation (20) is simplified to

$$\begin{aligned} \alpha \eta'''' + \eta'''' + \{u^2 + \Gamma + \Pi(1 - 2v\delta) - \gamma(I - \xi) - \zeta\xi\} \eta'' + (\gamma - \zeta) \eta' \\ + 2\sqrt{\beta} u \eta' + \ddot{\eta} + r w \rho d^2 \dot{\eta} - \frac{1}{2} C_D \rho d v^2 - \frac{1}{4} \pi C_m \rho d^2 \dot{v} = 0 \end{aligned} \tag{24}$$

The boundary conditions are simplified as

$$\begin{aligned} \eta'''(0) = -k_1 \eta(0), \eta'(0) = k_{T1} \eta'(0) \\ \eta'''(1) = k_2 \eta(1), \eta'(1) = -k_{T2} \eta'(1) \end{aligned} \tag{25}$$

The differential quadrature method is a standard numerical solution for solving ordinary and partial differential equations. The basic idea is to approximate the derivative of a particular point in the computational region concerning the spatial variables by a weighted sum of the function values of all points in the computational region [45]. It is assumed that the solution of Equation (24) is obtained in the form of as follows.

$$\eta(\xi, \tau) = \phi(\xi) \exp(\Omega\tau) \tag{26}$$

Substituting Equation (26) into Equations (24) and (25) yields

$$\begin{aligned} (1 + \alpha\Omega)\phi'''' + \{u^2 + \Gamma + \Pi(1 - 2v\delta) - \gamma(I - \xi) - \zeta\xi\} \phi'' \\ + (\gamma - \zeta + 2\sqrt{\beta} u \Omega) \phi' + (r w \rho d^2 \Omega + \Omega^2) \phi = 0 \end{aligned} \tag{27}$$

The boundary condition of the pipe is as follows.

$$\begin{aligned} \phi'''(0) = -k_1 \phi(0), \phi'(0) = k_{T1} \phi'(0) \\ \phi'''(1) = k_{T2} \phi(1), \phi'(1) = -k_{T2} \phi'(1) \end{aligned} \tag{28}$$

Then, the DQM discrete format of the control equation for the lateral vibration motion of the cold-water pipe under the action of the internal and external flow is as follows.

$$\sum_{j=1}^N \left\{ A_{ij}^{(4)} + [u^2 + \Gamma + \Pi(1 - 2\nu\delta) - \gamma(I - \xi_i) - \zeta\xi_i] A_{ij}^{(2)} + (\gamma - \zeta) A_{ij}^{(1)} \right\} \phi_j + \Omega \sum_{j=1}^N [(\alpha A_{ij}^{(4)} + 2\sqrt{\beta}u A_{ij}^{(1)}) + rwpd^2] \dot{\phi}_j + \Omega^2 \phi_i = 0 \tag{29}$$

The DQM discrete format of the boundary conditions is as follows.

$$\begin{aligned} \sum_{j=1}^N A_{1,j}^{(3)} + k_1 \phi_1 = 0, \quad \sum_{j=1}^N A_{2,j}^{(2)} \phi_j - k_{T1} \sum_{j=1}^N A_{2,j}^{(1)} \phi_j = 0, \\ \sum_{j=1}^N A_{N-1,j}^{(3)} \phi_j - k_2 \phi_{N-1} = 0, \quad \sum_{j=1}^N A_{N,j}^{(2)} \phi_j + k_{T2} \sum_{j=1}^N A_{N,j}^{(1)} \phi_j = 0. \end{aligned} \tag{30}$$

Use the subscript b to denote the boundary and d to denote the non-boundary, i.e.,

$$\phi_b = \{\phi_1, \phi_2, \phi_{N-1}, \phi_N\}^T, \quad \phi_d = \{\phi_3, \phi_4, \phi_{N-4}, \phi_{N-3}\}^T \tag{31}$$

Substituting Equation (31) into Equations (29) and (30), respectively, the following matrix form is obtained.

$$\Omega^2 I \phi_d + \Omega (G_{db} \phi_b + G_{dd} \phi_d) + (K_{db} \phi_b + G_{dd} \phi_d) = 0 \tag{32}$$

$$K_{bb} \phi_b + K_{bd} \phi_d = 0 \tag{33}$$

Solving Equation (33) using matrix operations and substituting the result into Equation (32) yields

$$G = -G_{db} K_{bb}^{-1} K_{bd} + G_{dd} \tag{34}$$

$$K = -K_{db} K_{bb}^{-1} K_{bd} + K_{dd} \tag{35}$$

Equation (33) has a non-zero solution of sufficient necessary conditions for the determinant of the matrix  $\Omega^2 I + \Omega G + K$  to be zero so that the eigenvalues of the transverse vibration equation of the pipe can be solved, combined with Mathematica software to write the program, and finite element software Ocrflex simulation, using the cold-water pipe parameters and fluid parameters as shown in Table 2, to analyze the characteristics of the dynamic response of the pipe.

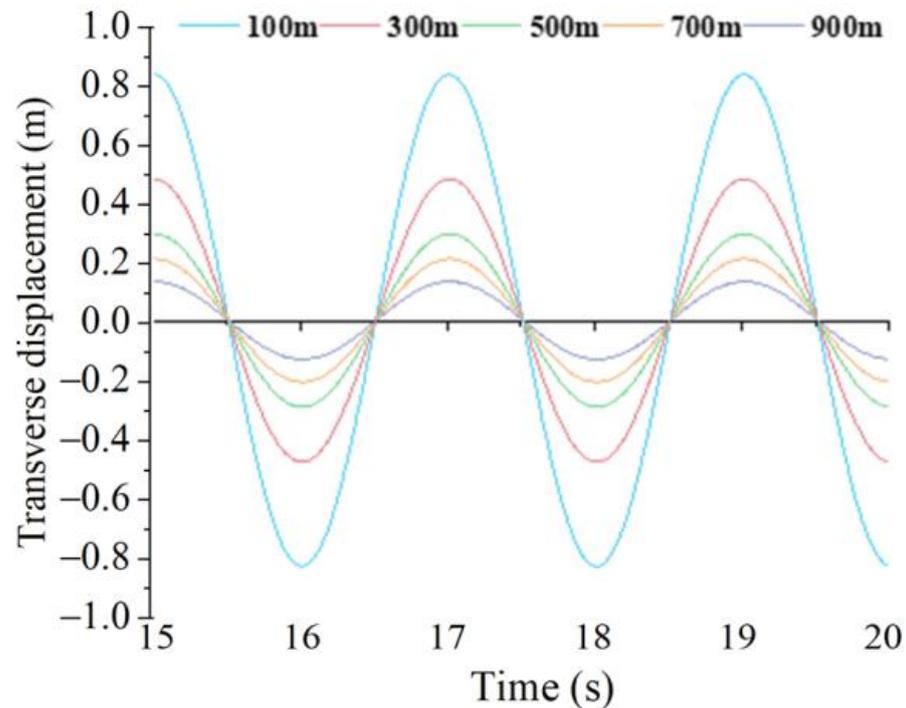
**Table 2.** Physical properties of CWP.

Property	Value
Bending stiffness (Gpa)	30
Inner diameter (m)	1.5
Outer diameter (m)	1.6
Density of the seawater (kg/m <sup>3</sup> )	1025
The density of the pipe (kg/m <sup>3</sup> )	1760
Pipe length (m)	1000
Axial equivalent tension	0
External flow velocity (m/s)	1.09
Velocity of the internal flow (m/s)	5.0
Top fluid pressure (Pa)	1.0 × 10 <sup>5</sup>
Poisson's ratio	0.3
Adapted drag coefficient	1.0
Additional mass coefficient	1.0

To verify the effectiveness of the DQM method, the software Ocrflex was used to calculate the transverse vibration amplitude of the cold-water pipe at water depths of 100 m, 300 m, 500 m, 700 m, and 900 m, and the results were compared with those solved using the DQM method, which are shown in Table 3 and Figure 5.

**Table 3.** Transverse vibration amplitude at different points of the pipe.

Pipe Section Location	Numerical Simulation Method (m)	DQM Method (m)
0.1	0.8536	0.8934
0.3	0.4952	0.4879
0.5	0.3016	0.2991
0.7	0.1954	0.2047
0.9	0.1187	0.1152



**Figure 5.** Lateral vibration amplitudes at different water depths (Ocrافlex).

As seen from Table 3, the results of the finite element of transverse vibration amplitude at different locations of the pipe are very close to the theoretical solution, proving the method’s effectiveness and correctness. As seen in Figure 5, the vibration response at the top of the pipe is the largest, and with the increase in water depth, the vibration response of the cold-water pipe keeps decreasing.

### 3. Results and Discussion

In the actual design process of the cold-water pipe project, three factors—waves, the clump weight, and internal flow velocity, respectively—significantly impact the lateral displacement and maximum bending moment of the cold-water pipe. This section will detail the impact of the appeal parameters on the mechanical properties of the chilled water pipe. The parameters shown in Tables 2 and 4 are still used in the analysis process.

**Table 4.** Environmental parameters of current and wave.

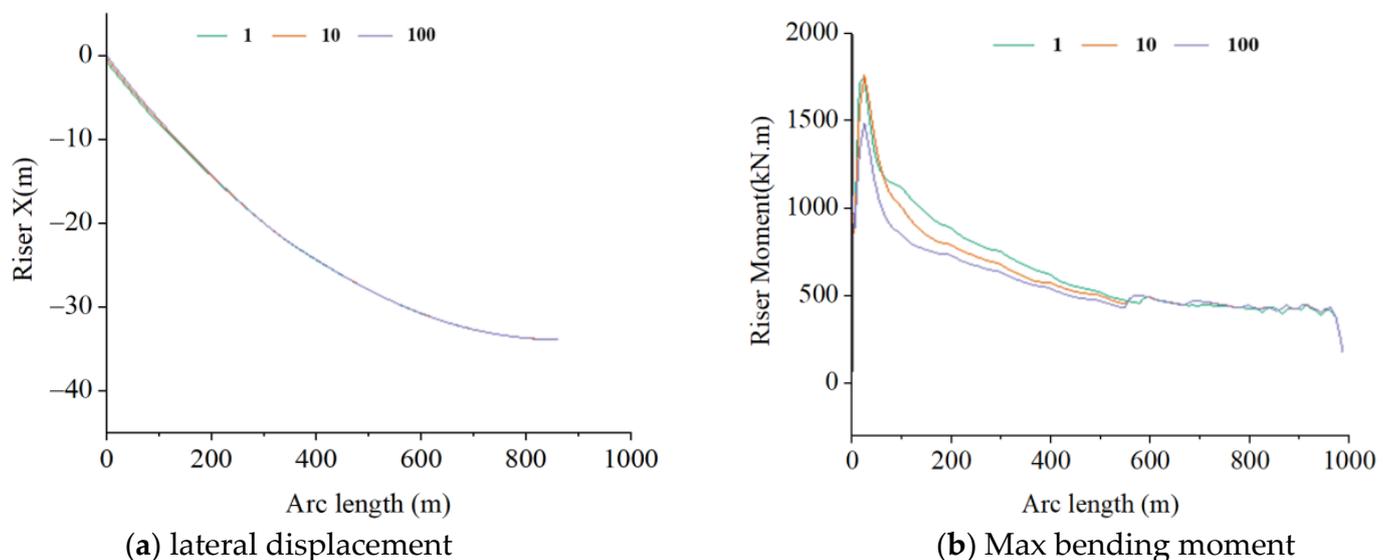
Periodicity (Year)		1	10	100
Wave	Significant wave height (m)	4.8	5.8	6.5
	Max wave height (m)	8.3	10.0	11.3
	Period (s)	7.8	9.0	9.8

### 3.1. Parameter Sensitivity Analysis

#### 3.1.1. Effects of Waves

The impact of waves on the cold-water pipe can be divided into two main aspects: one is the hydrodynamic load acting directly on the pipe; the other is the dynamic boundary conditions at the top of the pipe changed by acting on the floating body platform, while the movement of the floating body platform will also cause a more significant impact on the offset and vibration response of the pipe. In order to investigate the effect of waves on the lateral deflection and maximum bending moment of the pipe, three sets of wave parameters are selected for comparison and analysis in this subsection: 1-year, 10-year, and 100-year encounters.

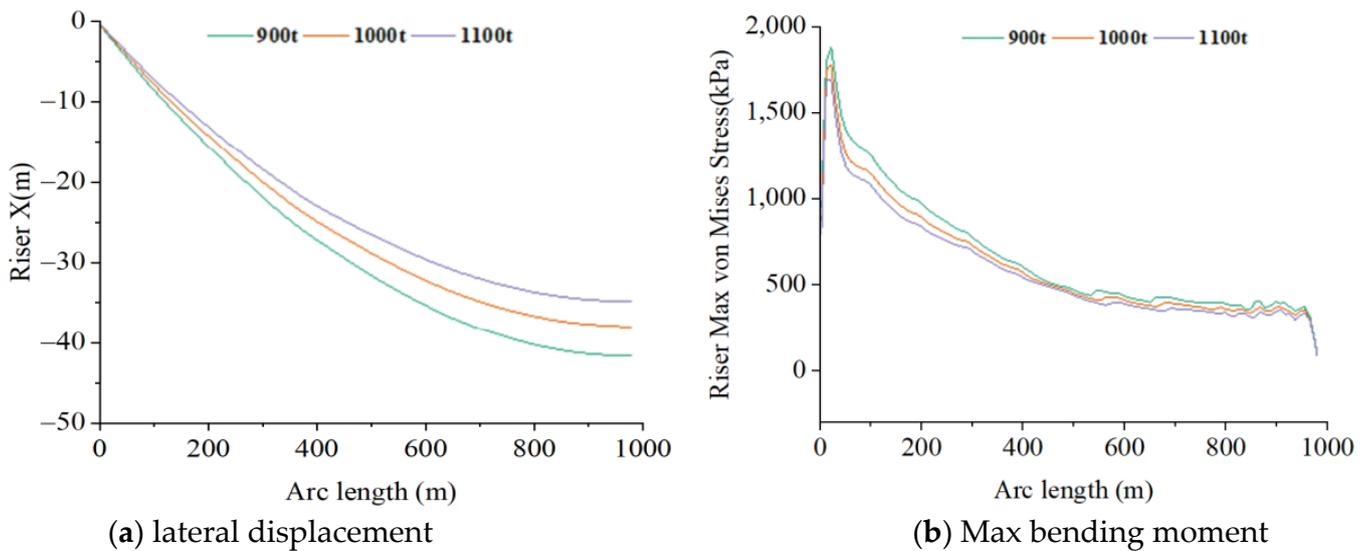
Figure 6a shows the variation of lateral deflection with wave intensity at different pipeline locations. It can be seen that the hydrodynamic load generated by the waves on the pipe gradually accumulates with the water depth. Thus, the lateral deflection of the pipe gradually increases, and its maximum can reach about 40 m. The lateral displacement of the pipe is almost constant under the three wave intensities, mainly because the pipe is a large-scale structure, and the wave parameters are not significantly different. The maximum bending moment development curve of the pipe considering only the wave strength is given in Figure 6b. It can be found that the wave strength has a more significant influence on the bending moment of the cold seawater pipe; when the wave strength is the same, with the increase in water depth, the bending moment of the pipe shows a trend of first increasing and then decreasing. The maximum bending moment appears in the upper part of the pipe, especially at the top, where the abrupt change of the maximum bending moment occurs. The larger the wave strength is, the more the abrupt change slightly decreases.



**Figure 6.** The lateral displacement and max bending moment of the pipe with respect to different wave intensities.

#### 3.1.2. Effects of the Clump Weight

This section focuses on the effect of the bottom counterweight block on the dynamic response of the cold seawater pipe. The counterweight weights of 900 t, 1000 t, and 1100 t are selected for comparative analysis, and other parameters are still selected from Table 4. Figure 7 shows the curves of the development of the pipe's lateral deflection and maximum bending moment with water depth under different counterweight weights.



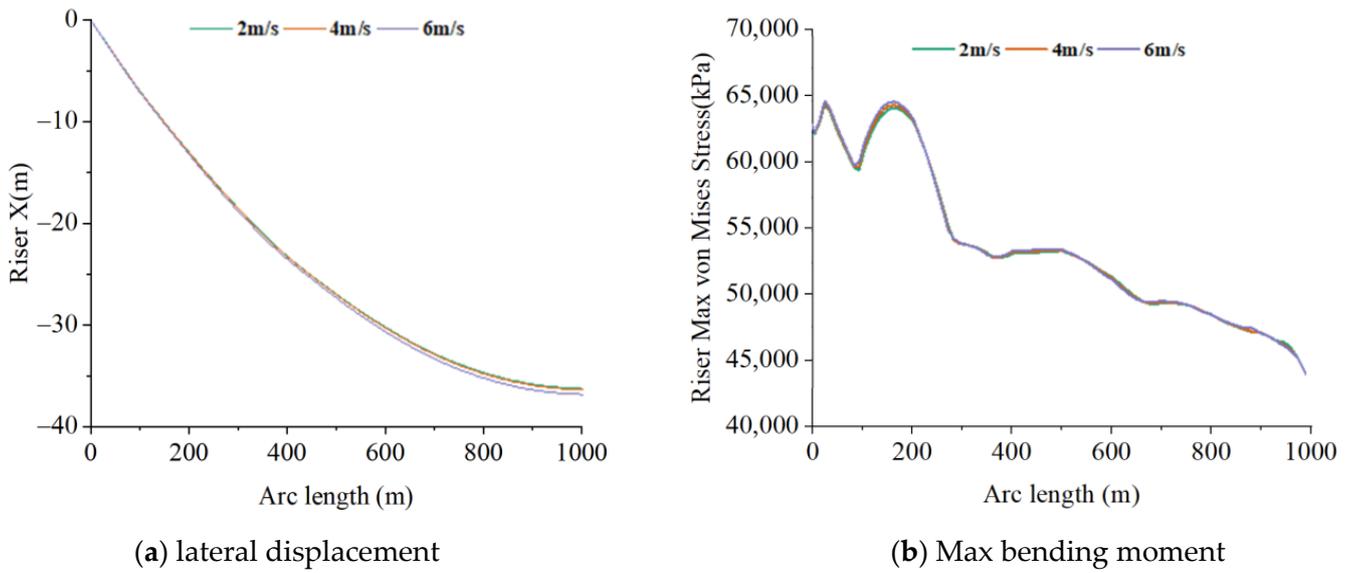
**Figure 7.** The transverse displacement and max bending moment of the pipe with respect to different clump weights at the bottom.

Figure 7a gives a graph of the development of the lateral offset of the pipe with the water depth at the different weights of the bottom counterweight. It can be found that with the increase in water depth, the lateral offset of the pipe gradually increases, and the growth of the lateral offset of the pipe tends to level off at 900–1000 m. With the increase in the weight of the bottom counterweight, the pipeline's lateral offset gradually decreases, indicating that the bottom counterweight can effectively reduce the lateral offset of the pipeline. According to Figure 7b, when the weight of the counterweight is 900 t, the maximum bending moment of the pipe increases sharply with the increase in water depth and then decreases gradually and levels off at 500–1000 m. When increasing the bottom counterweight's weight, the pipeline's maximum bending moment can be effectively reduced in the water depth range of 100–200 m.

### 3.1.3. Effects of Internal Flow Velocity

This section studies the effect of the internal flow velocity on the lateral deflection and maximum bending moment of the pipe. Three groups of internal flow velocities—2 m/s, 4 m/s, and 6 m/s—are selected, respectively, for comparison and analysis, while other parameters are still selected from Table 4. Figure 8 shows the curves of the development of lateral deflection and maximum bending moment of the pipe with water depth under different internal flow velocities.

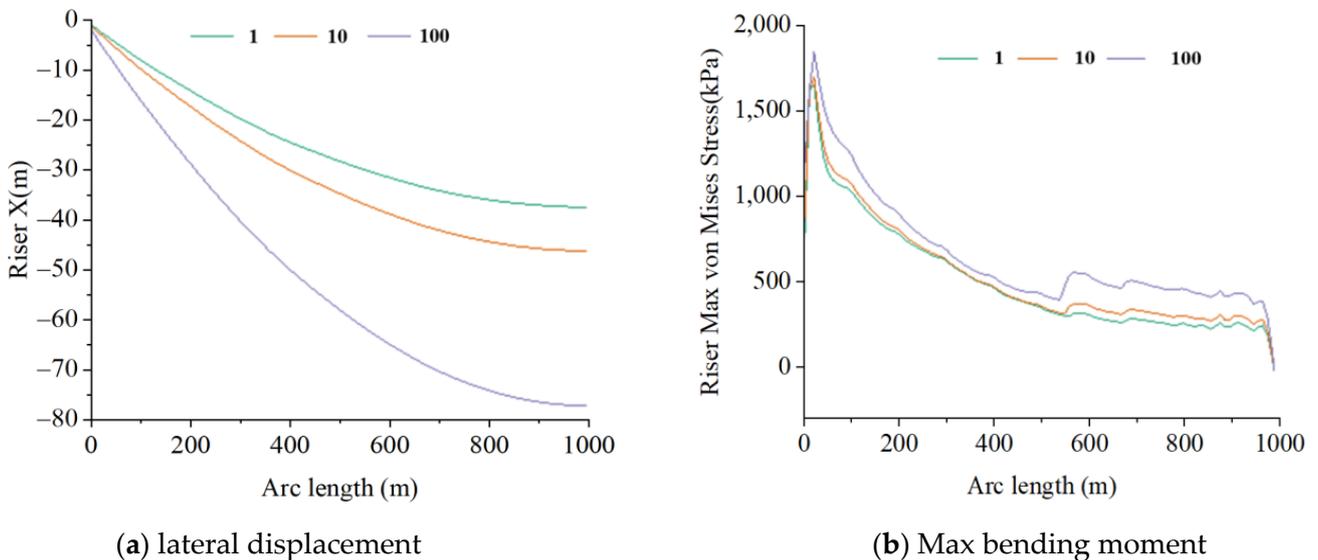
Figure 8a shows the development curve of lateral deflection of the pipe under the effect of internal flow only. It can be seen that the effect of the internal flow velocity on the lateral offset of the cold seawater pipe is negligible. With the increase in the water depth, the lateral offset increases, and the maximum offset of the cold seawater pipe under the three internal flow velocities are about 40 m, in line with the requirements of the actual project, the lateral offset of 40 m is not more than 5% of the water depth of 1200 m. Figure 8b is the development curve of the maximum bending moment of the pipe with the water depth under different internal flow velocities; the maximum bending moment of the cold seawater pipe is not significantly affected by the change of the internal flow velocity, and there is a sharp increase in the water depth of 30 m. The overall trend is that the maximum bending moment decreases gradually with the increase in water depth.



**Figure 8.** The transverse displacement and bending moment of the pipe with respect to the internal flow velocity.

### 3.1.4. Effects of Current Velocity

In order to investigate the effect of sea current on the lateral deflection and maximum bending moment of the pipeline, three sets of sea current flow parameters of a 1-year interval, 10-year interval, and 100-year interval are selected for simulation in this section. Figure 9 shows the graphs of the lateral deflection and maximum bending moment development of the pipeline under different sea current flow rates.



**Figure 9.** The transverse displacement and max bending moment of the pipe with respect to the external flow velocity.

From Figure 9a, it can be seen that the current velocity has a significant influence on the lateral offset of the cold-water pipe; with the increase in the current velocity, the lateral offset of the pipe increases. When the occasional one 100-year sea current is encountered, the lateral offset reaches 80 m, which exceeds 5% of the seawater depth at 1200 m and is not in line with the engineering reality, so special attention needs to be paid to the influence of the change of the current sea velocity on the lateral offset of the pipe. It can be seen from Figure 9b that the effect of sea current velocity changes on the maximum bending moment of the cold-water

pipe is mainly manifested in two aspects: one is the difference of the peak near the top of the cold seawater pipe, and the other is that the maximum bending moment of the pipe increases with the increase in current velocity, but its change trend is the same. The change in current velocity significantly impacts the maximum bending moment of the pipe.

### 3.2. Orthogonal Analysis of Key Parameters

The analysis in the previous section shows that the four factors of the wave, the clump weight, internal flow velocity, and current influence the mechanical properties of the pipe. In order to further analyze the three influencing factors on cold-water pipes, this subsection—through the orthogonal test design—offers a comprehensive analysis of the degree of their influence on cold-water pipes.

#### 3.2.1. Analysis of Orthogonal Experiments

For this multi-factor and multi-level analysis method of the number of simulation experiments leading into a power increase, the use of this comprehensive experimental approach requires a large number of simulation experiments; the workload is quite large, resulting in tedious work. In this paper, the orthogonal experiment method is widely used, and it is a feasible method to improve efficiency in seeking the optimal level of combination.

Through the analysis of the three factors in the previous section, to make the wave, the clump weight, internal flow velocity, and sea current as the dynamic characteristics of the cold-water pipe more significant, the four factors need to be observed to see their degree of influence on the dynamic response of the cold-water pipe. The orthogonal test matrix table with four factors and three levels was designed as shown in Table 5.

**Table 5.** CWP orthogonal test matrix table.

Horizontal	Factors			
	Wave (m)	Current (m/s)	Internal Flow Velocity (m/s)	The Clump Weight (t)
1	4.8	0.99	2	900
2	5.8	1.09	4	1000
3	6.5	1.42	6	1100

In analyzing the impact of different factors on the cold-water pipe, at the same time, consider each factor set as several parameters. The orthogonal  $3^4$  orthogonal test form was used to carry out the orthogonal test design, and the protocol was developed concerning the orthogonal form, as shown in Table 6.

**Table 6.** Orthogonal test table of CWP.

Number	Wave (m)	Current (m/s)	Internal Flow Velocity (m/s)	The Clump Weight (t)
1	4.8	0.99	2	900
2	4.8	1.09	4	1000
3	4.8	1.42	6	1100
4	5.8	0.99	4	1100
5	5.8	1.09	6	900
6	5.8	1.42	2	1000
7	6.5	0.99	6	1000
8	6.5	1.09	2	1100
9	6.5	1.42	4	900

#### 3.2.2. Simulation Data Analysis

In this subsection, the dynamic response analysis of the cold-water pipe was carried out using the finite element software Ocrflex simulation method, and nine numerical calculations were performed. The relevant data obtained are shown in Table 7.

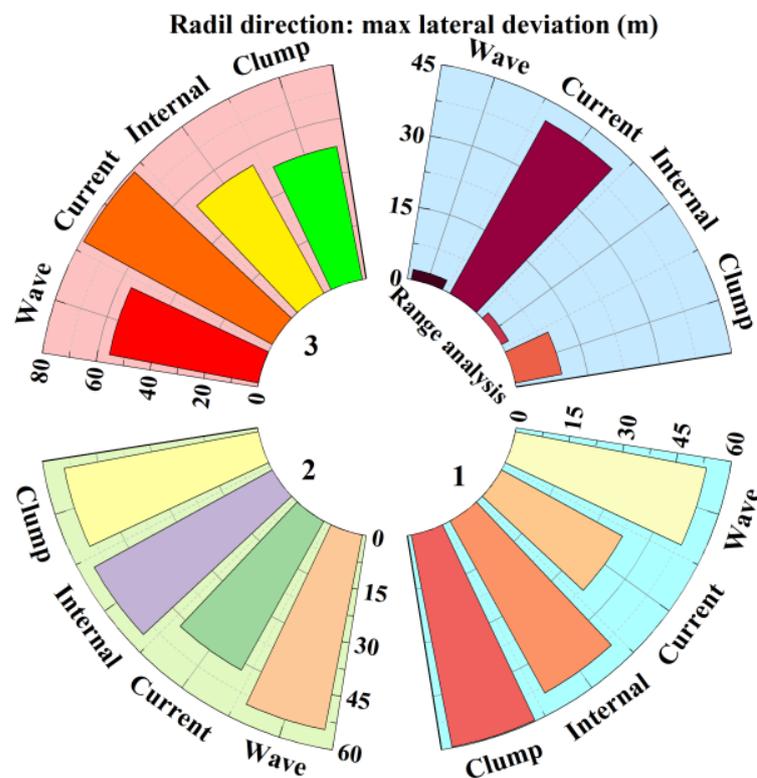
**Table 7.** Simulation data of nine groups CWP tests.

Numbers	Max Lateral Displacement (m)	Max Bending Moment (kN.m)
1	41.46	1854.8
2	46.8	1806.88
3	72.33	1910.79
4	34.73	1668.64
5	51.57	1892.18
6	78.35	1950.59
7	37.96	1446.81
8	42.88	1417.94
9	86.16	1840.65

### 3.2.3. Extremum Difference Analysis

In this subsection, four factors affecting the power response of the pipe and their three different levels are analyzed in an orthogonal simulation. In order to see the sensitivity of the different levels of different factors for the influence of the pipe dynamic response, the sensitivity is analyzed using the polar difference method. The basic principle of the polar difference method is divided into two steps: (1) calculate the average value of the same level under the conditions of different factors; (2) calculate the fluctuation range of the average value of the same factor under the conditions of different levels. The value of the fluctuation range is the resulting polar difference.

(1) The results of the maximum lateral displacement of the cold-water pipe obtained from the orthogonal experiment are post-processed to obtain the polar difference based on the maximum lateral displacement and plotted as shown in Figure 10. It can be seen that the order of importance affecting the lateral displacement of the cold-water pipe are current, the clump weight, wave, and internal flow; when the lateral offset of the cold-water pipe reaches the maximum, the parameters are as follows: wave is 6.5 m, current velocity is 1.42 m/s, the internal flow velocity is 5 m/s, and the clump weight is 900 t.



**Figure 10.** Range analysis of max lateral deviation range of CWP.

(2) The results of the maximum bending moment of the cold-water pipe obtained from the orthogonal experiment were post-processed to obtain the extreme difference based on the maximum bending moment and are plotted as shown in Figure 11. It can be seen that the order of importance of the maximum bending moment affecting the cold-water pipe are wave, sea current, the clump weight, and internal flow velocity; when the bending stress of the cold-water pipe reaches the maximum, the parameters are as follows: wave is 4.8 m, sea current is 1.42 m/s, the internal flow velocity is 5 m/s, and the clump weight is 900 t.

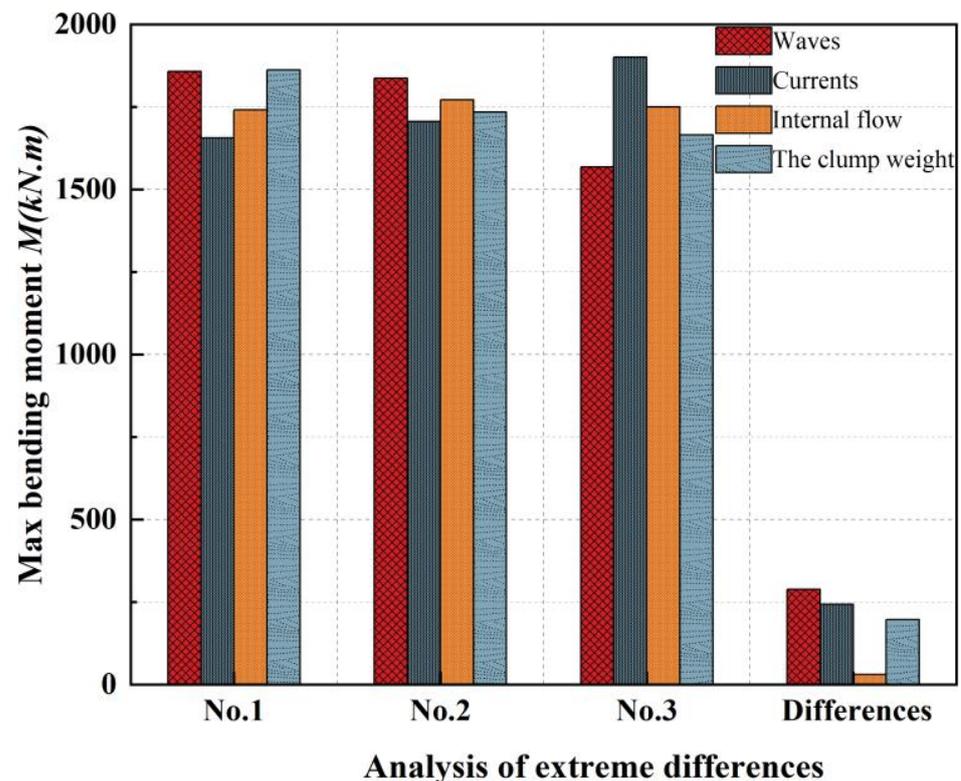


Figure 11. Range analysis max bending stress of CWP.

Through the above extreme difference analysis of the lateral offset and maximum bending moment of the cold-water pipe, it was concluded that the influence of wave order is in first place, which fully indicates that the change of wave has the most significant impact on the maximum bending moment of the cold-water pipe, which is the most critical factor affecting the dynamic response characteristics of the cold-water pipe; the change of the current velocity has the most apparent impact on the lateral offset of the cold-water pipe. In summary, it can be seen that the wave, the current velocity, and the clump weight at the bottom have a significant impact on the dynamic response of the cold-water pipe; therefore, in the design and manufacturing of the cold-water pipe, the installation and operation process need to pay attention to these three parameters.

#### 4. Conclusions

In this paper, the mechanical properties of ultra-large cold-water pipes for Ocean Thermal Energy Conversion are addressed. The lateral motion control equations of the pipe are derived based on the structural dynamics and semi-analytical solution methods, and the effect of the parameters of waves, the clump weight, internal flow, and sea currents are analyzed using finite element software. The specific conclusions are as follows:

(i). Based on the Euler–Bernoulli beam theory and differential quadrature method, the mechanical model of transverse vibration control of ultra-large cold-water pipe is established by comprehensively considering the effects of wave, sea current, internal flow, and the clump weight. The semi-analytical solution of the mechanical response of the pipe

is derived, and the correctness of the theoretical model is verified by comparing it with the numerical simulation results;

(ii). The wave strength, the clump weight, internal flow velocity, and sea current velocity can gradually increase the lateral deflection of the cold-water pipe, but different wave strength and internal flow velocity have little effect on the lateral deflection of the pipe. Increasing the clump weight at the bottom can effectively suppress the lateral deflection of the pipe, and the suppression effect is better near the location of the bottom counterweight; the larger the sea current velocity is, the larger the lateral deflection of the pipe is, which is likely to cause the cold-water pipe strength failure problem;

(iii). Wave strength, the clump weight, internal flow velocity, and sea current velocity have a greater effect on the maximum bending moment of the pipe, and their effects are first sharply increased, then gradually decreased until leveling off. However, the effect of the parameters is different in the location of the pipe section; at 100–200 m, the greater the wave strength, the smaller the maximum bending moment, and at 600 m, the maximum bending moment changes begin to level off. At 50 m–150 m, the clump weight at the bottom and the velocity of the current are larger; the maximum bending moment decreases gradually, and at 600 m, the maximum bending moment begins to level off;

(iv). Based on the orthogonal analysis of key parameters, the change of wave has the greatest influence on the maximum bending moment of cold seawater pipe, the change of sea current velocity has the most obvious influence on the lateral deflection of cold seawater pipe, and the clump weight at the bottom can effectively suppress the lateral deflection and maximum bending moment of pipe.

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